# TECH GUIDE 2.0E 2900Fb PWT LVL 

LIMIT STATES DESIGN HEADER \& BEAM

PWT FOCUSED ON EWP

# Designed to Outperform Traditional Lumber 

PWT ${ }^{\text {TM }}$ Laminated Veneer Lumber (LVL) is a vast improvement over traditional lumber.<br>Problems that naturally occur as sawn lumber dries - twisting, splitting, checking,<br>crowning and warping - are greatly reduced.

## Strength in Engineering

PWT LVL is made from ultrasonically and visually graded veneers arranged in a specific pattern to maximize the strength and stiffness of the veneers and to disperse the naturally occurring characteristics of wood, such as knots, that can weaken a sawn lumber beam. The veneers are then bonded with waterproof adhesives under pressure and heat. PWT LVL beams are exceptionally strong, solid and straight, making them excellent for most primary load-carrying beam applications.

## PWT LVL

## Available Grades and Sizes

PWT LVL is available in a range of depths and lengths, and is available in a standard thicknesses of 1-3/4". The 2.0E LVL is also available in a thickness of $1-1 / 2^{\text {" }}$ in some markets. In addition, a water-resistant coating is applied for extra weather protection during construction. Please verify availability with the PWT distributor in your area before specifying these products.

## Lifetime Limited Warranty

PWT products are backed by a lifetime limited warranty. Visit pwtewp.com or call (800) 515-7570 for a copy of the warranty.

## Software for Easy, Reliable Design

Our design/specification software enhances your in-house design capabilities. It offers accurate designs for a wide variety of applications with interfaces for printed output or plotted drawings. Through our distributors, we offer component design review services for designs using PWT products.

## Code Evaluation

PWT Laminated Veneer Lumber has been evaluated by CCMC for compliance with the National Building Code of Canada. Contact your local PWT distributor or visit pwtewp.com for the most current code reports.

- CCMC Evaluation Reports 13006-R and 11518-R
- APA Product Report ${ }^{\ominus}$ PR-L233C and PR-L280C


## Important Notes

1. PWT LVL shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent average moisture content in lumber will not exceed $15 \%$ nor a maximum of $19 \%$.
2. This guide is valid only for PWT LVL members supporting loads applied parallel to the face of the veneer ("edge" orientation).
3. The tables in this guide meet the design requirements of the National Building Code of Canada for Limit States Design and assume a normal importance category. Ensure that the specified design loads, duration of load increases and deflection limits that you use to select products from this guide are appropriate for your application and comply with local code requirements. If you do not know the correct design criteria and all the loads imposed on the component from all parts of the structure, seek qualified help from the architect, engineer or designer of the structure. Additional reference data on wood construction is available in the form of building codes, code evaluation reports and other design references.
4. The Quick Reference and Uniform Load Resistance (PLF) tables in this guide are only for uniform loads on simple (single) or equal, continuous (multiple)span members as noted in each table. For other conditions such as concentrated loads, unequal spans, etc., contact your PWT distributor.
5. Beam spans in this guide are typically measured from center-to-center of supports except for door and window headers. A structurally adequate bearing surface under the full width (thickness) of the member must be provided at each support.
6. Minimum bearing length is $1-1 / 2^{\prime \prime}$. At least one jack stud or cripple is required unless otherwise noted for a specific table. Refer to the Factored Reaction Resistance chart and the notes for each table. Verify local code requirements for minimum bearing.
7. Total load deflections are based on instantaneous loading. Long term deflection (creep) under sustained load has not been considered.
8. Vibration has not been considered in this guide. If PWT LVL is used as a floor joist, the designer shall perform the required vibration control checks.
9. PWT LVL is not cambered.
10. Higher grades of PWT LVL can be substituted for the indicated grade.
11. PWT LVL sized with the tables and design values in this guide requires continuous lateral restraint of the compression edge. Continuous restraint is defined as a maximum unbraced length of 24 ". This restraint is normally provided by sheathing and/or other framing members, which shall be adequately anchored to the LVL and the supporting structure. Framing conditions that do not provide continuous lateral restraint require special design. Contact your PWT distributor. Caution: Failure to provide adequate lateral restraint could result in an unstable member and reduce its load capacity.
12. Lateral restraint shall also be provided at all supports to prevent rotation or twisting.
13. Refer to the Connection Details page for information on designing nailed and bolted connections, minimum nail spacing and end distances, and for properly connecting multiple plies of LVL to form a built-up member.

## - <br> L V L

### 2.0E 2900 Fb PWTLVL

Standard Thickness of $1-3 / 4^{\prime \prime}$ (also available in $1-1 / 2^{\prime \prime}$ )
Standard Depths of $7-1 / 4^{\prime \prime}, 9-1 / 4^{\prime \prime}, 9-1 / 2^{\prime \prime}, 11-1 / 4^{\prime \prime}, 11-7 / 8^{\prime \prime}, 14^{\prime \prime}, 16^{\prime \prime}, 18^{\prime \prime}, 20^{\prime \prime}$, and $24^{\prime \prime}$

## Lengths up to 60 feet

Specific sizes may not be available in all locations, contact your local distributor for availability.
A water-resistant coating is applied to PWT LVL for extra weather protection during construction.
Code Evaluation Reports:

- CCMC Evaluation Reports 13006-R and 11518-R
- APA Product Report ${ }^{\text {P PR-L233C and PR-L280C }}$
or contact your local PWT distributor or visit pwtewp.com for the most current code reports.


### 2.0E 2900Fb PWT LVL Product Specifications \& Design Values

## SPECIFIED STRENGTHS \& STIFFNESS (PSI)

| Crade | 2.0 E 2900Fb |
| ---: | :--- |
| Bending, $\mathrm{Fb}^{4}$ | 5359 |
| Modulus of Elasticity, $\left.\mathrm{E}^{5} \times 10^{6} \mathrm{ps}\right)$ | 2.0 |
| Shear $\mathrm{Fv}_{\mathrm{v}}$ | 530 |
| Compression Stress Parallel to Grain, $\mathrm{Fc}^{2}$ | 5107 |
| Compression Stress Perpendicular to Grain, Fcp | 1365 |

## Notes:

1. PWT'm LVL shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent average moisture content in lumber will not exceed a yearly average of $15 \%$ and does not exceed $19 \%$ at any time.
2. The specified strengths and stiffness are for standard load duration. Specified strengths shall be adjusted according to code. Stiffness shall not be adjusted.
3. The specified strengths and stiffness are for members supporting loads applied parallel to the wide face ("edge" or "beam" orientation).
4. The specified Bending strength, fb, is tabulated for $12^{\prime \prime}$ depth. For depths greater than 12 ", multiply $\mathrm{f}_{\mathrm{b}}$ by (12/depth) ${ }^{0.20}$. For depths less than 12 ", multiply $\mathrm{f}_{\mathrm{b}}$ by ( 12 /depth) ${ }^{0.111}$. For depths less than $3-1 / 2^{1 "}$, multiply fb by 1.147 .
5. Deflection calculations shall include both bending and shear deformations.

Deflection for a simple span, uniform load: $\Delta=\frac{270 W L^{4}}{E b d^{3}}+\frac{28.8 w L^{2}}{E b d}$

$$
\begin{array}{rlrl}
\text { Where: } \Delta & =\text { deflection (in) } & E=\text { modulus of elasticity (from table) } \\
W & =\text { uniform load (pif) } & b=\text { width of beam (in) } \\
L=\operatorname{design~span~(ft)~} & d=\text { depth of beam (in) }
\end{array}
$$

Equations for other conditions can be found in engineering references.

## SECTION PROPERTIES AND ALLOWABLE CAPACITIES

| Depth | Weight (lb/ft) |  |  |  | Factored Moment ${ }^{2}(\mathrm{lb-ft})$ |  |  |  | Factored Shear (lb) |  |  |  | Moment of Inertia (in4) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-3/4" | 3-1/2' | 5-1/4" | 7" | 1-3/4" | 3-1/2' | 5-1/4" | 7" | 1-3/4" | 3-1/2' | 5-1/4" | 7" | 1-3/4" | 3-1/2' | 5-1/4" | 7" |
| 7-1/4" | 3.6 | 7.3 | 10.9 | 14.5 | 6516 | 13032 | 19548 | 26065 | 4035 | 8069 | 12104 | 16139 | 56 | 111 | 167 | 222 |
| 9-1/4" | 4.6 | 9.3 | 13.9 | 18.5 | 10324 | 20648 | 30973 | 41297 | 5148 | 10295 | 15443 | 20591 | 115 | 231 | 346 | 462 |
| 9-1/2" | 4.8 | 9.5 | 14.3 | 19.0 | 10857 | 21715 | 32573 | 43431 | 5287 | 10574 | 15860 | 21147 | 125 | 250 | 375 | 500 |
| 11-1/4" | 5.6 | 11.3 | 16.9 | 22.5 | 14943 | 29886 | 44830 | 59773 | 6261 | 12521 | 18782 | 25043 | 208 | 415 | 623 | 831 |
| 11-7/8" | 5.9 | 11.9 | 17.8 | 23.8 | 16550 | 33100 | 49650 | 66200 | 6608 | 13217 | 19825 | 26434 | 244 | 488 | 733 | 977 |
| $14 "$ | 7.0 | 14.0 | 21.0 | 28.0 | 22279 | 44558 | 66837 | 89116 | 7791 | 15582 | 23373 | 31164 | 400 | 800 | 1201 | 1601 |
| $16 "$ | 8.0 | 16.0 | 24.0 | 32.0 | 28332 | 56664 | 84997 | 113329 | 8904 | 17808 | 26712 | 35616 | 597 | 1195 | 1792 | 2389 |
| $18{ }^{\prime \prime}$ | 9.0 | 18.0 | 27.0 | 36.1 | 35023 | 70046 | 105070 | 140093 | 10017 | 20034 | 30051 | 40068 | 851 | 1701 | 2552 | 3402 |
| 20" | 10.0 | 20.0 | 30.0 | 40.1 | 42337 | 84674 | 127011 | 169348 | 11130 | 22260 | 33390 | 44520 | 1167 | 2333 | 3500 | 4667 |
| 24 " | 12.0 | 24.0 | 36.1 | 48.1 | 58782 | 117565 | 176347 | 235130 | 13356 | 26712 | 40068 | 53424 | 2016 | 4032 | 6048 | 8064 |

Notes:

1. The Factored Moment and Shear are for standard load duration and shall be adjusted according to code.
2. The tabulated Factored Moment Resistance assumes continuous lateral support of the compression edge. For other conditions, multiply the Factored Moment Resistance by the beam lateral stability factor, KL, as defined in the CSA 086.
3. The $3-1 / 2^{\prime \prime}, 5-1 / 4 "$ and 7 " beam widths listed above can be either a single piece or a combination of widths. For example, a 7 " wide beam may be a single billet beam of 7 ", two plies of $3-1 / 2^{"}$, a single $1-3 / 4$ " attached to a $5-1 / 4$ " billet beam, a $3-1 / 2^{" 1}$ with a $1-3 / 4$ " ply attached to each face, or four plies of $1-3 / 4$ ". Refer to the Connection Assemblies details on page 14 for additional information.
4. The tabulated weight is an estimate and shall only be used for design purposes. Contact PWT for actual shipping weights.

## Fasteners:

Refer to pages 14-15 for information on connecting multiple plies and for the equivalent specific gravity for design of nailed and bolted connections.

## FACTORED REACTION RESISTANCE (LBS)

| Bearing Length |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Width | 1-1/2' | 2" | 2-1/2' | 3" | 3-1/2 ${ }^{\text {² }}$ | 4" | 4-1/2 ${ }^{\text {² }}$ | 5" | 5-1/2' | 6 C | 6-1/2' | 7" | 7-1/2 ${ }^{\text {² }}$ | 8" | 8-1/2 ${ }^{\text {² }}$ | 9" | 9-1/2' | 10" | 10-1/2" | 11" | 11-1/2" | 12" |
| 1-3/4" | 2866 | 3822 | 4777 | 5733 | 6688 | 7644 | 8599 | 9555 | 10510 | 11466 | 12421 | 13377 | 14332 | 15288 | 16243 | 17199 | 18154 | 19110 | 20065 | 21021 | 21976 | 22932 |
| 3-1/2" | 5733 | 7644 | 9555 | 11466 | 13377 | 15288 | 17199 | 19110 | 21021 | 22932 | 24843 | 26754 | 28665 | 30576 | 32487 | 34398 | 36309 | 38220 | 40131 | 42042 | 43953 | 45864 |
| 5-1/4" | 8599 | 11466 | 14332 | 17199 | 20065 | 22932 | 25798 | 28665 | 31531 | 34398 | 37264 | 40131 | 42997 | 45864 | 48730 | 51597 | 54463 | 57330 | 60196 | 63063 | 65929 | 68796 |
| $7{ }^{7}$ | 11466 | 15288 | 19110 | 22932 | 26754 | 30576 | 34398 | 38220 | 42042 | 45864 | 49686 | 53508 | 57330 | 61152 | 64974 | 68796 | 72618 | 76440 | 80262 | 84084 | 87906 | 91728 |

Notes:

1. Tabulated values are based on the factored compression resistance, perpendicular-to-grain, of the LVL. This is suitable for beams bearing on steel or the end-grain of studs.
2. Verify that the support for the beam is structurally adequate to carry the reaction. The compressive resistance, parallel-to-grain, of studs may require more studs than the bearing length above indicates.
3. For beams bearing on wood plates, the required bearing length will increase based on the bearing resistance (compression perpendicular-to-grain) of the species and grade used for the plate material.
4. Verify local code requirements concerning minimum bearing.

## Quick Reference Tables Beam with Floor Loading

## Table Usage:

1. Select the correct table for the supported floor joist condition (simple or continuous).
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

Example: A beam with a $16^{\prime}$ span carries $15^{\prime}-0$ " simple span joists on each side.
Solution: Using the Simple-Span Floor Joists table with $30^{\prime}-0$ " span carried, select either $3-1 / 2^{\prime \prime} \times 16$ " or $5-1 / 4$ " x 14 ".
Note: The $3-1 / 2^{\prime \prime} \times 16^{\prime \prime}$ requires $7-1 / 2^{\prime \prime}$ intermediate bearing if a multiple span beam.


CONTINUOUS FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | 26' | 28' | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" |
| 12'0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | 14 " | $14 "$ | 14 " | - |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | $14 "$ | 14 " | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | 16 | - | - | - | - |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 16'0" | 3-1/2" | $14^{\prime \prime}$ | $16 "$ | 16 " | $16 "$ | 16 " | - | - | - | - | - | - |
|  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14{ }^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ |
| 18'0" | 3-1/2" | $16 "$ | $16 "$ | $18{ }^{\prime \prime}$ | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 16" | $16 "$ | $16 "$ | 18" |
| 20'0" | 3-1/2" | 18" | 18 " | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" | - |
| 22'0" | 3-1/2" | " | " | 18 | 18 | 1 | 18 | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
| 24-0" | 3-1/2" | 18 | 18 | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $18{ }^{\prime \prime}$ | 18" | - | - | - | - | - | - | - | - | $\cdot$ |

SIMPLE-SPAN FLOOR JOISTS (SPECIFIED FLOOR LOADS: 40 PSF LIVE LOAD, 15 PSF DEAD LOAD)

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | $26^{\prime}$ | $28^{\prime}$ | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12'0" | 3-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | 14 " | $14 "$ | $14 "$ | 14 " | $14{ }^{\prime \prime}$ | $14 "$ | 14 " |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 11-7/8" | 11-7/8" |
| 16'0" | 3-1/2" | $14 "$ | $14 "$ | $14 "$ | $14{ }^{\prime \prime}$ | $16 "$ | $16 "$ | 16 " | $16 "$ | 16 | 16 | - |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 " | 14 " | $14 "$ |
| 18'0" | 3-1/2" | $16 "$ | $16 "$ | $16{ }^{\prime \prime}$ | 16 " | 16 " | 16 " | 18" | 18" | - | - | - |
|  | 5-1/4" | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 " | $14{ }^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ |
| 20'0" | 3-1/2' | $16 "$ | 18 " | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  | 5-1/4" | 14 " | $16^{\prime \prime}$ | 16 " | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" |
| 22'0" | 3-1/2' | 18 " | 18" | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18 " | 18" | 18" | 18" | 18" |
| 24'-0" | 3-1/2" | 18 | $1{ }^{\prime \prime}$ | 18 | $1{ }^{\prime \prime}$ | 18 | 18 | - | - | - | - | - |
|  | 5-1/4" | 18" | 18 " | $18{ }^{\prime \prime}$ | 18" | 18 " | 18" | - | - | - | - | - |

## Notes:

1. Use the Continuous Floor Joists table where the floor joists are continuous (multiple span) over the beam. Use the Simple-Span Floor Joists table where the floor joists frame into the side of or end on top of the beam.
2. Span is center-to-center of supports and is valid for simple and equal, continuous beam spans.
3. End supports require 3 " bearing. Interior supports require 6 " bearing except $7-1 / 2^{\prime \prime}$ is required where bold. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
4. Deflections are limited to $\mathrm{L} / 360$ live load and $\mathrm{L} / 240$ total load.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages 14-15 for connection details, or use the Exacte by PWT software for muttiple-ply member connections
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. A floor live load reduction factor for tributary area has been applied where applicable per NBCC.
9. Do not use where marked "-".


## Quick Reference Tables

## Beam with Combined Loading <br> Table Usage:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

Example: A beam with a 9'-6" span supports a $32^{\prime}-0$ " span carried for a 20 psf Roof Live load.
Solution: Using the correct table for the roof load with $32^{\prime}-0$ " span carried, select either $3-1 / 2^{\prime \prime} \times 11-1 / 4^{\prime \prime}$ or $5-1 / 4^{\prime \prime} \times 9-1 / 4^{\prime \prime}$.
Specified Loads


ROOF - 20 PSF SNOW OR LIVE, 15 PSF DEAD | FLOOR - 40 PSF LIVE, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | 26' | $28^{\prime}$ | 30' | 32' | 34' | $36^{\prime}$ | 38' | $40^{\prime}$ |
| 6'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" |
| 9'6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
| 12'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | 14 " | $14 "$ | 14 " | 14 " |
|  | 5-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'0" | 3-1/2" | 14 " | 14 " | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $16 "$ | $16 "$ | 16 | $16 "$ | $16 "$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14 " | 14 " | 14 " |
| 16'0" | 3-1/2" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 16 " | 18" | 18" | 18" | 18" | 18" |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | 14 " | 14 " | $16 "$ | $16 "$ | 16 " | $16 "$ |
| 16'6" | 3-1/2" | $16 "$ | $16^{\prime \prime}$ | $16 "$ | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14 "$ | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ |
| 18'-0" | 3-1/2" | $16 "$ | $18^{\prime \prime}$ | 18" | 18" | 18 " | 18 " | - | - | - | - | - |
|  | 5-1/4" | 14 " | $16^{\prime \prime}$ | $16 "$ | $16 "$ | 16 " | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" |
| 18'6" | 3-1/2" | 18 " | $18{ }^{\prime \prime}$ | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18 " | 18" | 18" | - | - |
| 20'0" | 3-1/2" | $18{ }^{\prime \prime}$ | - | - | 18 | " | " | " | " | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" | - | - | - |
| 22'0" | 3-1/2" | 18 | " | " | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | - | - | - | - | - | - | - | - |
| 24'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | . | - | . |

ROOF - 30 PSF SNOW, 15 PSF DEAD \| FLOOR - 40 PSF LIVE, 15 PSF DEAD


## Quick Reference Tables

## Beam with Combined Loading

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

Example: A beam with a 9'-6" span supports a $32^{\prime}$-0" span carried for a 40 psf Roof Snow load.
Solution: Using the correct table for the roof load with 32'-0" span carried, select either 3-1/2" x $11-1 / 4$ " or $5-1 / 4$ " x $9-1 / 4$ ".
Specified Loads


ROOF - 40 PSF SNOW, 15 PSF DEAD \| FLOOR - 40 PSF LIVE, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | $26^{\prime}$ | 28' | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 9'6" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | - | - | - |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
| 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14 "$ |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 12'0" | 3-1/2" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14" | 14 " | 14" | $14 "$ | $16 "$ | $16 "$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " |
| 14'-0" | 3-1/2" | $14 "$ | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | - | - |
|  | 5-1/4" | 11-7/8" | 14 " | 14 " | 14 " | 14 " | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | $16 "$ | $16 "$ |
| 16'0" | 3-1/2" | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | 14 " | $16 "$ | 16 " | $16{ }^{\prime \prime}$ | 16 " | 16 " | 16" | 16 " | 18" | 18" |
| 16'6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | . | - | - | - | - | - |
| 18'0" | 3-1/2" | 18" | 18" | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" | - | - |
| 18'6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | 18 " | 18 " | - | - | - | - | - | - | - |
| 20'0" | 3-1/2" | 18 | 18 | 18" | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | - | - | - | - | - | - | - | - |
| 22'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
| 24'0" | $3-1 / 2^{\prime \prime}$ | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

ROOF - 50 PSF SNOW, 15 PSF DEAD \| FLOOR - 40 PSF LIVE, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $20^{\prime}$ | 22' | 24' | 26' | $28^{\prime}$ | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 9'6" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | - | - | - | - | - |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 10'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14" | 14 " | 14 " | 14 " |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 12'0" | 3-1/2" | 14 " | 14 " | 14 " | $14{ }^{\prime \prime}$ | $14 "$ | $14 "$ | $16 "$ | $16 "$ | $16 "$ | 16 " | - |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | $14 "$ | $14 "$ |
| 14'-0" | 3-1/2" | 16 " | $16^{\prime \prime}$ | $16^{\prime \prime}$ | 16 | $16 "$ | 18 " | - | - | - |  | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | 14 " | 16 | 16" | $16 "$ | 16" | 16" |
| 16'0" | 3-1/2" | 18" | 18" | 18" | 18" | - | - | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $16 "$ | $16 "$ | 16 " | 16 " | 16" | 18" | 18" | 18" | 18" | 18" |
| 16'6" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16^{\prime \prime}$ | - | - | - | - | - | - | . |
| 18'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
| 18'6" | 3-1/2" | 18 | 18 " | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | - | - | - | - | - | - | - | - | - |
| 20'0" | 3-1/2" | $1{ }^{1}$ | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | - | - | - | - | - | - | - | - | - | - |
| 22'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |
| 24'-0" | $3-1 / 2^{\prime \prime}$ | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## Notes:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3" bearing except $4-1 / 2$ " is required where bold. The end supports for the standard garage door spans of $9^{\prime}-6,{ }^{\prime}$ " $16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-6^{\prime \prime}$ are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads include 100 plf for an exterior wall and assume a 2 ' maximum overhang on the roof and an interior support at mid-span of the floor joists.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages 14-15 for connection details, or use the Exacte by PWT software for multiple-ply member connections.
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. A floor live load reduction factor for tributary area has been applied where applicable per NBCC.
9. Do not use where marked "-"

## Quick Reference Tables

## Beam with Roof Loading <br> Table Usage:

1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

Example: A beam with a $16^{\prime}-6 "$ span supports a $36^{\prime}-0$ " span carried for a 30 psf Roof Snow load.
Solution: Using the correct table for the roof load with $36^{\prime}-0^{\prime \prime}$ span carried, select either $3-1 / 2^{\prime \prime} \times 16$ " or $5-1 / 4^{\prime \prime} \times 14^{\prime \prime}$.
Specified Loads


ROOF - 20 PSF SNOW OR LIVE, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | $26^{\prime}$ | $28^{\prime}$ | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 9'6" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 10'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12'0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 14'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | $14 "$ |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 16'0" | 3-1/2" | 11-7/8" | 11-7/8" | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ | $14 "$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 16'6" | 3-1/2" | 11-7/8" | $14 "$ | 14 " | 14 " | 14 " | 14 " | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16 "$ | $16 "$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 18'0" | 3-1/2" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16 "$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | $14^{\prime \prime}$ | 14 " | $14 "$ | 14 " | 14 " | 14 " |
| 18'6" | 3-1/2" | $14 "$ | 14 | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18 " |
|  | 5-1/4" | 11-7/8" | 11-7/8" | 14 " | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ |
| 20'-0" | 3-1/2" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18 " | 18 " | 18 " | 18 " | 18 " |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $16 "$ | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ |
| 22'0" | 3-1/2" | $16^{\prime \prime}$ | $16 "$ | 18 " | 18 " | 18 " | 18 " | 18 " | - | I | " | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 16 " | $16 "$ | $16 "$ | 16 " | $16 "$ | $16 "$ | 18" | 18" | 18" |
| 24'-0" | 3-1/2" | $18{ }^{\prime \prime}$ | 18 " | 18 " | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 181 | 18" | 181 | 18" | - |

ROOF - 30 PSF SNOW, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | 26' | 28 ' | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'-0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 9'6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12'-0" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | $14 "$ | $14 "$ | 14 " | $14 "$ | $14 "$ |
|  | 5-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" |
| 16'0" | 3-1/2" | 14 | 14 " | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14 "$ | $16 "$ | $16 "$ | $16 "$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | $14 "$ | 14 " | 14 " | 14 " | 14 " | 14 " |
| 16'6" | 3-1/2" | 14 " | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | 16 " | $16 "$ | - |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | 14 " | $14 "$ |
| 18'0" | 3-1/2" | $14^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 18 " | $18{ }^{\prime \prime}$ | 18 " | 18" |
|  | 5-1/4" | $14 "$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $16 "$ | 16 " | $16 "$ | $16 "$ |
| 18'6" | 3-1/2" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $18{ }^{\prime \prime}$ | 18 " | 18 " | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | 16 " | $16 "$ | 16 " | $16 "$ | $16 "$ |
| 20'0" | 3-1/2" | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | 18 " | $18{ }^{\prime \prime}$ | 18 " | 18 " | 18" | $1{ }^{\prime \prime}$ | - | 1 | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" |
| 22'0" | 3-1/2" | 18 " | 18 " | 18 " | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18 " | 18" | 18" | 18" | - |
| 24'-0" | 3-1/2" | 18 | 181 | 18 | 18 | 18 | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | - | - | - | - | . | - |

## Notes:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3 " bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-66^{\prime \prime}, 16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-66^{\prime \prime}$ are limited to 3 " (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $L / 360$ live/snow load and $L / 240$ total load
4. Loads assume a 2 ' maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages 14-15 for connection details, or use the Exacte by PWT software for multiple-ply member connections.
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. Pitch adjusted roof dead load shall not exceed 15 psf.
9. Do not use where marked "-".

## Quick Reference Tables

 Beam with Roof Loading1. Select the correct table for the roof loads needed.
2. Choose the required center-to-center span for the beam in the Span column.
3. Select the span carried by the beam across the top of the table.
4. Read the beam size or choice of beam sizes from the table.

Example: A beam with a $16^{\prime}$-6" span supports a $36^{\prime}-0$ " span carried for a 50 psf Roof Snow load.
Solution: Using the correct table for the roof load with 36'0" span carried, select a $5-1 / 4^{\prime \prime} \times 16^{\prime \prime}$. Note: A $3-1 / 2^{\prime \prime}$ beam does not work.
Specified Loads


ROOF - 40 PSF SNOW, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | 26' | $28^{\prime}$ | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 9'6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 10'-0" | 3-1/2" | 9-1/4" | $9-1 / 4 "$ | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 12'0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14 "$ | 14 " | 14 " | 14 " |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
| 14'-0" | 3-1/2" | 11-7/8" | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14{ }^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $14 "$ | $16 "$ | 16 " | 16 " |
|  | 5-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" | 14 " | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " |
| 16'0" | 3-1/2" | $14{ }^{\prime \prime}$ | $14^{\prime \prime}$ | $16 "$ | $16 "$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16{ }^{\prime \prime}$ | $16 "$ | $18^{\prime \prime}$ | 18 " | 18 " |
|  | 5-1/4" | 11-7/8" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | 14 " | 14 " | $16 "$ | $16 "$ |
| 16'6" | 3-1/2" | $14 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | - | - | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | 14 " | $14{ }^{\prime \prime}$ | 14 " | $14 "$ | 14 " | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16^{\prime \prime}$ |
| 18'0" | 3-1/2" | $16 "$ | $16^{\prime \prime}$ | 18 " | 18 " | 18 " | 18" | 18" | 18 " | - | - | - |
|  | 5-1/4" | $14^{\prime \prime}$ | $14^{\prime \prime}$ | $14{ }^{\prime \prime}$ | $16^{\prime \prime}$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" |
| 18'-6" | 3-1/2" | $16 "$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | " | - | - | - | - |  |  |
|  | 5-1/4" | 14 " | $14 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" |
| 20'0" | 3-1/2" | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | $18{ }^{\prime \prime}$ | 16 | " | " | " | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16^{\prime \prime}$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" | - |
| 22'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | 18" | 18" | 18" | - | - | . | - | - | - |
| 24'-0" | 3-1/2" | 18 | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $18{ }^{\prime \prime}$ | . | - | - | - | - | - | - | - | - | - |

ROOF - 50 PSF SNOW, 15 PSF DEAD

| Span | Beam Width | Span Carried By Beam |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 20' | 22' | 24' | 26' | $28^{\prime}$ | 30' | 32' | 34' | 36' | 38' | 40' |
| 6'0" | 3-1/2" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" |
| 8'-0" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
|  | 5-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 7-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" |
| 9'6" | 3-1/2" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" |
| 10'-0" | 3-1/2" | 9-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" |
|  | 5-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/4" | 9-1/2" | 9-1/2" | 11-1/4" | 11-1/4" |
| 12'0" | 3-1/2" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 14 " | 14 " | 14 " | 14 " | $14{ }^{\prime \prime}$ | $14 "$ | 14 " |
|  | 5-1/4" | 9-1/2" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | 11-7/8" |
| 14'0" | 3-1/2" | $14 "$ | $14{ }^{\prime \prime}$ | $14 "$ | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 16 | $16 "$ | $16^{\prime \prime}$ |
|  | 5-1/4" | 11-1/4" | 11-7/8" | 11-7/8" | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | 14 " | $14^{\prime \prime}$ | 14 " | 14 " | 14 " |
| 16'0" | 3-1/2" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" | - |
|  | 5-1/4" | $14^{\prime \prime}$ | 14 " | 14 " | $14^{\prime \prime}$ | 14 " | 16 " | 16 " | 16 " | 16 " | 16 " | $16 "$ |
| 16'6" | 3-1/2" | $16^{\prime \prime}$ | $16 "$ | $16 "$ | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $14{ }^{\prime \prime}$ | 14 " | 14 " | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | - |
| 18'0" | 3-1/2" | 18 " | 18 " | 18" | 18" | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | 18" | 18" |
| 18'6" | 3-1/2" | 18 " | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16 "$ | $16 "$ | $16 "$ | $16 "$ | 18" | 18" | 18" | 18" | - | - | - |
| 20'0" | 3-1/2" | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | $16^{\prime \prime}$ | 18" | 18" | 18" | 18" | 18" | - | - | - | - | - |
| 22'0" | 3-1/2" | " | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | 18" | 18" | - | - | - | - | - | - | - | - | - |
| 24'-0" | $3-1 / 2^{\prime \prime}$ | - | - | - | - | - | - | - | - | - | - | - |
|  | 5-1/4" | - | - | - | - | - | - | - | - | - | - | - |

## Notes:

1. Span is center-to-center of supports and is valid for simple beam spans only.
2. End supports require 3 " bearing except $4-1 / 2^{\prime \prime}$ is required where bold. The end supports for the standard garage door spans of $9^{\prime}-66^{\prime \prime}, 16^{\prime}-66^{\prime \prime}$ and $18^{\prime}-6 "$ are limited to $3^{\prime \prime}$ (two trimmers) on each end. The bearing length is based on the compressive resistance, perpendicular-to-grain, of the LVL. See the Factored Reaction Resistance table on page 4 for additional information.
3. Deflections are limited to $\mathrm{L} / 360$ live/snow load and $\mathrm{L} / 240$ total load.
4. Loads assume a 2 ' maximum overhang on the roof.
5. Beam width can be either a single piece of LVL or built up from multiple plies that are nailed, bolted or connected with other approved fasteners. Refer to pages 14-15 for connection details, or use the Exacte by PWT software for multiple-ply member connections.
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. Pitch adjusted roof dead load shall not exceed 15 psf.
9. Do not use where marked "-".

Table Usage:

1. Select the span required.
2. Divide the design loads by the number of plies to verify each ply of the member. Divide the design loads by 2 to verify a $3-1 / 2^{"}$ width or by 3 to verify a $5-1 / 4^{\prime \prime}$ width.
3. Compare the factored design total load to the Factored Total Resistance column.
4. Compare the unfactored design total load to the Total Load Deflection Resistance.
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deflection limit. 6. Select a product that satisfies all three conditions.

Example:
For a 16 ' beam span, select a 2 - and 3 -ply beam that satisfies an L/360 Live Load deflection limit for the following specified loads: Live Load = 480 plf; Dead Load $=180$ plf
Calculate Design Loads:
Factored Total Load $=(1.5 \times 480)+(1.25 \times 180)=945 \mathrm{plf}$
Unfactored Total Load $=480+180=660$ plf

Solution for a 2 -ply beam:
Factored Total Load per ply
Unfactored Total Load per ply $=660 / 2=330 \mathrm{plf}$
Unfactored Live Load per ply $=480 / 2=240 \mathrm{plf}$
Use 2 plies $1-3 / 4^{\prime \prime} \times 14^{4}$

## Solution for a 3 -ply beam:

| Factored Total Load per ply | $=945 / 3=315 \mathrm{plf}$ |
| :--- | :--- |
| Unfactored Total Load per ply | $=660 / 3=220 \mathrm{plf}$ |
| Unfactored Live Laad per ply | $=480 / 3=160 \mathrm{plf}$ |
| Use2 plies $1-3 / 4^{\times 1} \times 11 / 7 / 8$ |  |


| $\begin{gathered} \text { Span } \\ (\mathrm{ft}) \end{gathered}$ | 1-3/4" $\times 7-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 2^{\prime \prime}$ |  |  |  | 1-3/4" $\times 11-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 11-7 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 14{ }^{\text {" }}$ |  |  |  | 1-3/4" $\times 16{ }^{\text {" }}$ |  |  |  | 1-3/4" $\times 18$ " |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance |  |  |  |
|  | Live <br> L/480 | Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \\ & 0 \text { L/2 } 20 \end{aligned}$ |  |  | Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \\ & 1 / 240 \end{aligned}$ | 0 | Live | Live Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ |  |  | e Load | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { Total } \\ \text { Load } \\ \hline \text { LL/240 } \end{array} \\ \hline \end{array}$ |  |  | ve Load | $\begin{array}{\|c\|} \hline \begin{array}{c} \text { Total } \\ \text { Load } \end{array} \\ \hline 0 \mathrm{~L} / 240 \end{array}$ |  |  | ive Load <br> 180 L/360 | $\begin{aligned} & \text { Total } \\ & \text { Load } \\ & 0 / 240 \end{aligned}$ |  |  |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \\ & 0 \text { L/2 } \end{aligned}$ |  | Live | Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \\ & 0 \text { L/240 } \end{aligned}$ |  |
| 6 | 494 | 659 | 985 | 1071 | 947 | 1263 |  | 1366 | 1014 | 141353 |  | 1403 | 1553 |  |  | 1662 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | 323 | 430 | 642 | 917 | 630 | 840 |  | 1170 | 677 | 7902 |  | 1202 | 1054 | 541406 |  | 1423 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | 221 | 295 | 439 | 802 | 438 | - 584 | 872 | 1023 | 471 | 1628 | 938 | 1051 | 744 | 44992 |  | 1245 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9 | 158 | 211 | 313 | 639 | 316 | 421 | 627 | 909 | 340 | - 454 | 676 | 933 | 542 | 723 | 1079 | 1105 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | 116 | 155 | 230 | 516 | 235 | 313 | 465 | 817 | 253 | 3337 | 502 | 839 | 406 | 542 | 807 | 994 | 471 | 1628 | 937 | 1049 | 735 | 35980 |  | 1237 | 1042 | 1390 |  | 1414 | 1404 |  |  | 1591 |
| 11 | 88 | 118 | 173 | 426 | 179 | 238 | 353 | 676 | 193 | 3257 | 381 | 711 | 311 | 1415 | 618 | 903 | 362 | 362483 | 719 | 953 | 569 | 569759 |  | 1124 | 813 | 1084 |  | 1285 | 1104 |  |  | 1445 |
| 12 | 68 | 91 | 133 | 357 | 139 | 186 | 274 | 567 | 150 | 150200 | 296 | 597 | 244 | 4325 | 482 | 823 | 284 | 284379 | 562 | 873 | 449 | 49599 | 891 | 1030 | 645 | 860 |  | 1177 | 882 | 1176 |  | 1324 |
| 13 | 54 | 72 | 105 | 303 | 110 | 147 | 216 | 482 | 119 | 9159 | 234 | 508 | 194 | 4259 | 383 | 700 | 226 | 226302 | 447 | 776 | 360 | 350480 | 713 | 950 | 520 | 693 | 1032 | 1085 | 714 | 952 |  | 1221 |
| 14 | 43 | 58 | 83 | 261 | 89 | 119 | 173 | 415 | 96 | 128 | 188 | 437 | 157 | 17209 | 309 | 602 | 183 | 83 244 | 361 | 668 | 292 | 292390 | 578 | 881 | 424 | 566 | 841 | 1007 | 585 | 780 |  | 1133 |
| 15 | 35 | 47 | 67 | 227 | 73 | 97 | 141 | 361 | 78 | 105 | 153 | 380 | 128 | 8171 | 252 | 524 | 150 | 50200 | 295 | 581 | 241 | 241321 | 475 | 783 | 350 | 467 | 693 | 939 | 485 | 647 | 961 | 1057 |
| 16 | - | - | - |  | 60 | 80 | 116 | 316 | 65 | 87 | 125 | 333 | 107 | 142 | 208 | 459 | 125 | 5166 | 244 | 509 | 200 | 200 267 | 394 | 687 | 292 | 390 | 577 | 875 | 406 | 542 | 804 | 990 |
| 17 | - | - |  |  | 50 | 67 | 96 | 280 | 54 | 72 | 104 | 294 | 89 | 119 | 173 | 406 | 104 | 4139 | 204 | 450 | 168 | 168225 | 330 | 607 | 246 | 329 | 485 | 774 | 343 | 458 | 678 | 931 |
| 18 | - | - | - | - | 42 | 57 | 80 | 249 | 46 | 61 | 87 | 262 | 75 | 101 | 146 | 361 | 88 | 118 | 171 | 401 | 143 | 143190 | 279 | 541 | 209 | 279 | 411 | 689 | 292 | 390 | 576 | 853 |
| 19 |  |  |  |  | 36 | 48 | 68 | 223 | 39 | 52 | 74 | 234 | 64 | 86 | 124 | 324 | 75 | 101 | 145 | 359 | 122 | 122163 | 238 | 484 | 179 | 239 | 351 | 617 | 251 | 335 | 493 | 764 |
| 20 | - | - | - | - | 31 | 41 | 58 | 200 | 33 | 45 | 63 | 211 | 55 | 74 | 105 | 291 | 65 | 87 | 124 | 323 | 105 | 105140 | 204 | 436 | 155 | 207 | 302 | 556 | 217 | 289 | 425 | 689 |
| 21 |  | - |  |  |  |  |  |  |  | - |  |  | 48 | 64 | 91 | 264 | 56 | 75 | 107 | 292 | 91 | 1122 | 176 | 395 | 134 | 179 | 261 | 503 | 189 | 252 | 369 | 624 |
| 22 | - | - | - | - |  |  |  |  |  | - |  |  | 42 | 56 | 78 | 239 | 49 | 65 | 92 | 266 | 80 | \% 106 | 153 | 359 | 118 | 157 | 227 | 458 | 165 | 220 | 322 | 567 |
| 23 | - | - | - | - |  | - | - | - |  |  |  |  |  |  |  |  | 43 | 57 | 80 | 242 | 70 | 0 93 | 133 | 328 | 103 | 138 | 199 | 418 | 145 | 194 | 282 | 518 |
| 24 | - | - | - | - |  | - | - | - |  | - | - |  |  |  |  |  | 38 | 51 | 70 | 222 | 62 | 282 | 117 | 300 | 91 | 122 | 175 | 383 | 128 | 171 | 248 | 475 |
| 25 | - | - | - | - |  |  |  |  |  |  | - |  |  |  |  |  | 33 | 45 | 61 | 204 | 55 | 573 | 103 | 276 | 81 | 108 | 154 | 352 | 114 | 152 | 220 | 437 |
| 26 | - | - | - | - |  |  |  |  |  | - |  |  |  |  |  |  | 30 | 40 | 54 | 188 | 49 | 965 | 91 | 254 | 72 | 96 | 137 | 325 | 102 | 136 | 195 | 403 |
| 27 | - | - | - | - |  |  | - |  |  | - | - | - |  |  |  |  |  |  |  |  | 43 | 358 | 80 | 235 | 65 | 86 | 121 | 300 | 91 | 122 | 174 | 373 |
| 28 | - | - | - | - |  | - | - | - |  | - | - | - |  |  |  |  |  |  |  |  | 39 | 952 | 71 | 218 | 58 | 77 | 108 | 279 | 82 | 109 | 155 | 346 |
| 29 | - | - | - | - | - | - | - | - | - | - | - | - |  |  |  |  |  |  | - |  | 35 | 54 | 64 | 203 | 52 | 70 | 97 | 259 | 74 | 99 | 139 | 321 |
| 30 | - | - | - | - |  | - | - | - | - | - | - |  |  |  |  |  |  | - | - |  | 32 | 42 | 57 | 189 | 47 | 63 | 87 | 241 | 67 | 89 | 125 | 300 |

## Design Assumptions:

1. Span is the center-to-center distance of the supports and is valid for simple or equal, continuous span applications.
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard (100\%) duration and is adjusted to account for the selfweight of the member. The specified dead load shall not exceed the specified live load.
4. Live Load Deflection Resistance is limited to $\mathrm{L} / 360$ or $\mathrm{L} / 480$ as noted in the table. Vibration has not been considered.
5. Total Deflection Resistance is limited to $\mathrm{L} / 240$. Long term deflection (creep) has not been considered.
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 4.

## Additional Notes:

1. The tabulated resistances represent the capacity of the member in pounds per lineal foot (pif) of length.
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns.
3. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design.
4. For $1-3 / 4$ " thick LLL, depths of 16 " and greater shall be used with a minimum of two plies unless designed specificiclly asa single ply with proper Iateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined.
5. The tabulated resistances in the tables are for a single ply of $1-3 / 4$ " LLL. For $33-1 / 2$ " wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4$ " wide member, divide the design loads by 3.
6. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages 14-15, or use the Exacte by PWT software for multiple-ply member connections.
7. Do not use a product where designated ""." without further analysis by a design professional.

## ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | 10' | 12' | 14' | 16' | 18' | $20^{\prime}$ | 22' | 24' | 26' | 28' | $30^{\prime}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L/480 | 1/4" | 5/16" | 3/8" | 3/8" | 7/16" | 1/2" | 9/16" | 5/8" | 5/8" | 11/16" | 3/4" |
| L/360 | 5/16" | 3/8" | 7/16" | 9/16" | 5/8" | 11/16" | 3/4" | 13/16" | 7/8" | 15/16" | $1{ }^{\prime \prime}$ |
| L/240 | 1/2" | 5/8" | 11/16" | 13/16" | 7/8" | $1{ }^{\prime \prime}$ | 11/8" | 1-3/16" | 1-5/16" | 1-3/8" | $1-1 / 2^{\prime \prime}$ |

${ }^{*}$ Deflections rounded to the nearest $1 / 16^{\prime \prime}$.
To review PWT LVL products, please visit pwtewp.com.

Table Usage:

1. Select the span required. For beams with a pitch of $2: 12$ or greater, the horizontal span shall be muttipied by the appropriate roof pitch adiustment factor from the table at the bottom of this page.
2. Divide the design loads by the number of plies to verify each ply of the member. Divide the design loads by 2 to verify a $3-1 / 2^{2}$ width or by 3 to verify $a 5-1 / 4^{\text {" }}$ width.
3. Compare the fatcored design total load to the Factored Total Resistance column.
4. Compare the unfactored design total load to the Total Load Deflection Resistance.
5. Compare the unfactored design live load to the Live Load Deflection Resistance for the appropriate deffection limit. For a live load deflection limit of L/480, compare the unfactored design live load to the L/480 Live Load Deflection Resistance from the Uniform Floor Load Resistance Tables.
6. Select a product that satisfies all three conditions.

Note: The serviceability limit states Importance Factor for Snow Load, Is, of 0.9 can be applied to the specified snow loads for
evaluation of the deflection resistance. See the example to the right.

Example:
For an 10' horizontal beam span with a pitch of 4:12, select a 2- and 3-ply beam that satisfies an L/240 Snow Load deflection limit for the following specified loads: Snow Load = 720 plf; Dead Load = 400 plf.

Calculate Beam Span: $10^{\prime} \times 1.054=10.54^{\prime}->$ Use Span $=11^{\prime}$

Factored Total Load Unfactored Total Load Unfactored Snow Load

Solution for a 2-ply beam:

Factored Total Load per ply Unfactored Total Load per ply Unfatotored Snow Load per ply<br>$=1580 / 2=790 \mathrm{plf}$ $=1048 / 2=524 \mathrm{plf}$ $=648 / 2=324 \mathrm{plf}$ Use 2 plies $1-3 / 4^{\prime \prime} \times 111-1 / 4^{4}$

$=(1.5 \times 720)+(1.25 \times 400)=1580 \mathrm{plf}$
$=(0.9 \times 720)+400=1048 \mathrm{plf}$
$=0.9 \times 720=648 \mathrm{plf}$

## Solution for a 3-ply beam:

| Factored Total Load per ply | $=1580 / 3=527$ plf |
| :--- | :--- |
| Unfactored Total Load per ply | $=1048 / 3=350 \mathrm{plf}$ |
| Unfactored Snow Load per ply | $=648 / 3=216$ plf |
| Use 3 plies $1-3 / 4^{\prime \prime} x 9-1 / 4^{"}$ |  |


| $\begin{aligned} & \text { Span } \\ & (\mathrm{ft}) \end{aligned}$ | 1-3/4" $\times 7-1 / 4{ }^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 4^{\prime \prime}$ |  |  |  | 1-3/4" $\times 9-1 / 2^{\prime \prime}$ |  |  |  | 1-3/4" $\times 11-1 / 4$ " |  |  |  | 1-3/4" $\times 11-7 / 8^{\prime \prime}$ |  |  |  | 1-3/4" $\times 14{ }^{\prime \prime}$ |  |  |  | 1-3/4" $\times 16{ }^{\text {" }}$ |  |  |  | 1-3/4" $\times 181$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl.Resistance |  |  |  | Unfactored Defl. Resistance $\qquad$ |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance $\qquad$ |  |  |  | Unfactored Defl. Resistance |  |  |  | Unfactored Defl. Resistance 픙 |  |  |  | Unfactored Defl. Resistance |  |  |  |
|  | $\begin{array}{\|c} \hline \text { Snow/ } \\ \text { Loa } \end{array}$ | $\begin{aligned} & \text { //Live } \\ & \text { oad } \end{aligned}$ | $\begin{array}{\|c} \text { Total } \\ \text { Load } \end{array}$ |  |  | ow/Live Load |  |  |  | now/Live Load |  | 㤟部 |  | ow/Live Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ |  |  | ow/Live Load | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ |  |  | $\begin{aligned} & \text { ow/Live } \\ & \text { Load } \end{aligned}$ | $\begin{array}{\|l\|} \hline \text { Total } \\ \text { Load } \end{array}$ |  |  | $\begin{aligned} & \text { ow/Live } \\ & \text { Load } \end{aligned}$ |  |  | $\begin{array}{\|c} \text { Snow } \\ \text { Loo } \end{array}$ | w/Live oad | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ |  |
|  | L/360 | L/240 | L/180 |  |  | L 60 L/240 | L/180 |  |  | L60 L/240 | L/180 |  | L/360 | L L/240 | L/180 |  | L/360 | L50 L240 | L/180 |  | L/360 | 50 L/240 L | L/180 |  | L/360 | L50 L/240 | L/180 |  | L/360 | L/240 | L/180 |  |
| 6 | 659 | 989 |  | 1071 | 1263 | 63 |  | 1366 | 1353 | 53 |  | 1403 |  |  |  | 1662 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | 430 | 646 | 857 | 917 | 840 | 40 |  | 1170 | O 902 | 02 |  | 1202 | 1406 |  |  | 1423 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 | 295 | 443 | 587 | 802 | 584 | 884 876 |  | 1023 | 3628 | 28943 |  | 1051 | 992 |  |  | 1245 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9 | 211 | 316 | 418 | 639 | 421 | 21632 | 838 | 909 | 454 | 54681 | 903 | 933 | 723 | 1085 |  | 1105 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 | 155 | 233 | 308 | 516 | 313 | 213 470 | 622 | 817 | 337 | 37506 | 671 | 839 | 542 | 2813 |  | 994 | 628 | 893 |  | 1049 | 980 |  |  | 1237 | 1390 |  |  | 1414 |  |  |  | 1591 |
| 11 | 118 | 177 | 232 | 426 | 238 | 338358 | 473 | 676 | 257 | 257386 | 510 | 711 | 415 | 5623 | 826 | 903 | 483 | 725 |  | 953 | 759 |  |  | 1124 | 1084 |  |  | 1285 |  |  |  | 1445 |
| 12 | 91 | 137 | 179 | 357 | 186 | 186279 | 367 | 567 | 200 | 200 301 | 397 | 597 | 325 | 5488 | 645 | 823 | 379 | 568 | 752 | 873 | 599 | 988 |  | 1030 | 860 |  |  | 1177 | 1176 |  |  | 1324 |
| 13 | 72 | 108 | 141 | 303 | 147 | 147221 | 290 | 482 | 159 | 159239 | 314 | 508 | 259 | 9388 | 513 | 700 | 302 | 2453 | 598 | 776 | 480 | 720 |  | 950 | 693 | 31040 |  | 1085 | 952 |  |  | 1221 |
| 14 | 58 | 87 | 113 | 261 | 119 | 19178 | 233 | 415 | 128 | 128193 | 252 | 437 | 209 | 9314 | 413 | 602 | 244 | - 367 | 483 | 668 | 390 | - 585 | 773 | 881 | 566 | 6849 |  | 1007 | 780 |  |  | 1133 |
| 15 | 47 | 71 | 91 | 227 | 97 | 97146 | 190 | 361 |  | 105157 | 205 | 380 | 171 | 1257 | 338 | 524 | 200 | - 301 | 395 | 581 | 321 | 1482 | 635 | 783 | 467 | 7701 | 927 | 939 | 647 | 970 |  | 1057 |
| 16 | 39 | 59 | 75 | 199 | 80 | 80120 | 156 | 316 | 87 | 87130 | 169 | 333 | 142 | 212 | 279 | 459 | 166 | ¢ 250 | 327 | 509 | 267 | 1401 | 528 | 687 | 390 | 0585 | 772 | 875 | 542 | 813 |  | 990 |
| 17 | 32 | 49 | 62 | 175 | 67 | 67101 | 130 | 280 | 72 | 22109 | 141 | 294 | 119 | 9179 | 233 | 406 | 139 | 209 | 274 | 450 | 225 | 237 | 443 | 607 | 329 | 9493 | 650 | 774 | 458 | 687 | 907 | 931 |
| 18 | - | - | - | - | 57 | 5785 | 109 | 249 | 61 | 6192 | 118 | 262 | 101 | 1151 | 196 | 361 | 118 | 177 | 231 | 401 | 190 | -286 | 374 | 541 | 279 | 9419 | 551 | 689 | 390 | 585 | 771 | 853 |
| 19 | - | - |  |  | 48 | 8872 | 92 | 223 | 52 | 5278 | 100 | 234 | 86 | 129 | 167 | 324 | 101 | 151 | 196 | 359 | 163 | 3245 | 319 | 484 | 239 | 359 | 471 | 617 | 335 | 502 | 661 | 764 |
| 20 | - | - | - |  | 41 | 162 | 78 | 200 | 45 | 4567 | 85 | 211 | 74 | 111 | 143 | 291 | 87 | 130 | 168 | 323 | 140 | 211 | 274 | 436 | 207 | 7310 | 406 | 556 | 289 | 434 | 570 | 689 |
| 21 | - | - | - |  | 36 | 3654 | 67 | 181 | 39 | 3958 | 73 | 191 | 64 | 96 | 123 | 264 | 75 | 113 | 145 | 292 | 122 | 2183 | 237 | 395 | 179 | 9269 | 351 | 503 | 252 | 378 | 495 | 624 |
| 22 | - | - | - | - | 31 | 3147 | 58 | 164 | 34 | 3451 | 63 | 173 | 56 | 84 | 106 | 239 | 65 | 98 | 125 | 266 | 106 | 6160 | 206 | 359 | 157 | 7236 | 306 | 458 | 220 | 331 | 432 | 567 |
| 23 | - | - | - | - |  |  |  |  |  | - - | - |  |  |  |  |  | 57 | 86 | 109 | 242 | 93 | 140 | 180 | 328 | 138 | 8207 | 268 | 418 | 194 | 291 | 379 | 518 |
| 24 | - | - | - | - | - | - - | - | - |  | - - | - | - | - | - | - |  | 51 | 76 | 96 | 222 | 82 | 124 | 158 | 300 | 122 | 2183 | 236 | 383 | 171 | 257 | 334 | 475 |
| 25 | - | - | - |  |  |  |  |  |  | - - | - |  |  |  |  |  | 45 | 67 | 84 | 204 | 73 | 110 | 139 | 276 | 108 | 8162 | 209 | 352 | 152 | 229 | 296 | 437 |
| 26 | - | - | - | - |  | - - | - | - |  | - - | - | - | - |  |  |  | 40 | 60 | 74 | 188 | 65 | 98 | 123 | 254 | 96 | 145 | 185 | 325 | 136 | 204 | 263 | 403 |
| 27 | - | - | - | - | - | - - | - | - |  | - - | - | - | - | - |  |  | 36 | 54 | 66 | 174 | 58 | 87 | 110 | 235 | 86 | 130 | 165 | 300 | 122 | 183 | 235 | 373 |
| 28 | - | - | - | - |  | - | - | - |  | - - | - |  |  |  |  |  | 32 | 48 | 58 | 161 | 52 | 78 | 98 | 218 | 77 | 116 | 147 | 279 | 109 | 164 | 210 | 346 |
| 29 | - | - |  |  |  |  |  |  |  | - - | - |  |  |  |  |  |  |  |  |  | 47 | 71 | 87 | 203 | 70 | 105 | 132 | 259 | 99 | 148 | 189 | 321 |
| 30 | - | - | - | - |  |  |  |  |  | - | - |  |  |  |  |  |  |  |  |  | 42 | 64 | 78 | 189 | 63 | 95 | 119 | 241 | 89 | 134 | 170 | 300 |

## Design Assumptions:

1. Span is the center-to-center distance of the supports, along the sloped length of the member and is valid for simple or equal, continuous span applications.
2. The values in the tables are for uniform loads only.
3. Factored Total Resistance is for standard $(100 \%)$ duration and is adjusted to account for the self-weight of the member. The specified dead load shall not exceed the specified live load.
4. Live Load Deflection Resistance is limited to $\mathrm{L} / 360$ or $\mathrm{L} / 240$ as noted in the table.
5. Total Deflection Resistance is limited to $\mathrm{L} / 180$. Long term deflection (creep) has not been considered.
6. These tables assume full lateral support of the compression edge. Other conditions require further analysis by a design professional.
7. Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
8. Proper bearing must be provided. Bearing length must be checked for support reactions with the table on page 4.

## PITCH ADJUSTMENT

| Pitch | $2: 12$ | $3: 12$ | $4: 12$ | $5: 12$ | $6: 12$ | $7: 12$ | $8: 12$ | $9: 12$ | $10: 12$ | $11: 12$ | $12: 12$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |


| Factor | 1.014 | 1.031 | 1.054 | 1.083 | 1.118 | 1.158 | 1.202 | 1.25 | 1.302 | 1.357 | 1.414 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

## Additional Notes:

1. The tabulated resistances represent the capacity of the member in pounds per lineal foot (plf) of length.
2. The designer shall check the Factored Total Resistance, the Total Deflection Resistance and the appropriate Live Load Deflection Resistance columns.
3. For beams with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate pitch adjustment factor from the table above.
4. Where the Deflection Resistance is blank, the Factored Total Resistance governs the design.
5. For $1-3 / 4$ " thick LVL, depths of 16 " and greater shall be used with a minimum of two plies unless designed specifically as a single ply with proper lateral bracing, such as a marriage beam for each half of a manufactured home before the units are joined.
6. The tabulated resistances in the tables are for a single ply of $1-3 / 4^{" ~ L V L . ~ F o r ~ a ~} 3-1 / 2$ " wide member, divide the design loads by 2 to verify the resistance of each ply. For a $5-1 / 4^{" 1}$ wide member, divide the design loads by 3 .
7. The member width shall be properly built up by connecting plies of the same grade of LVL. Refer to the multiple-ply connections on pages 14-15, or use the Exacte by PWT software for multiple-ply member connections.
8. Do not use a product where designated "." without further analysis by a design professional.

## Temporary Bracing \& Warnings



## Warning The following conditions are NOT permitted!

Do not use visually damaged products without first checking with your local PWT Engineered Wood Products distributor or sales office.


All notched or drilled beams must be reviewed by a professional engineer. See hole detail on page 13 for allowable hole sizes and locations.

Do not notch beam at support.

## Installation Details

| P3 BEAM CONNECTION <br> Hanger shall apply load equally to each ply or special design required |  |
| :---: | :---: |
|  | (11) WINDOW/DOOR HEADER |
| (12) WINDOW/DOOR HEADER | (@3) beam hole detalls |
| (Q4) MASONRY HANGER <br> Note: Protect wood from contact with concrete as required by code. | Notes: <br> 1. These guidelines apply to uniformly loaded beams selected from the Quick Reference Tables or the Uniform Load Tables or designed with PWT's design/specification software only. For all other applications, such as beams with concentrated loads, please contact your Pacific Woodtech distributor for assistance. <br> 2. Round holes can be drilled anywhere in "Area A" provided that: no more than four holes are cut, with the minimum spacing described in the diagram. The maximum hole size is $1-1 / 2^{\prime \prime}$ for depths up to $9-1 / 4$, ," and 2 " for depths greater than $9-1 / 4^{\prime \prime}$. <br> 3. Rectangular holes are NOT allowed. <br> 4. DO NOT drill holes in cantilevers without prior approval from the project designer. <br> 5. Other hole sizes and configurations MAY be possible with further engineering analysis. For more information, contact your Pacific Woodtech distributor. <br> 6. Up to three $3 / 4$ " holes may be drilled in "Area $\mathbf{B}$ " to accommodate wiring and/or water lines. These holes shall be at least 12 " apart. The holes shall be located in the middle third of the depth, or a minimum of 3 " from the bottom and top of the beam. For beams shallower than $9-1 / 4^{\prime \prime}$, locate holes at mid-depth. <br> 7. Protect plumbing holes from moisture. |

## Connection of Multiple Ply Beams


(P2)
TOP-LOADED BEAM -
BOLTED CONNECTION (SEE CONNECTION ASSEMBLIES FOR MORE DETAILS)
 ASTM grade A-307 (or better)
bolts. Use washers on both faces
Framing is applied to top of the beam so that each ply carries an equal load

SIDE-LOADED BEAM
(SEE CONNECTION ASSEMBLIES FOR MORE DETAILS)


SIDE LOADS ARE NOT RECOMMENDED FOR BEAMS OVER
5-1/2" WIDE UNLESS EQUALLY APPLIED TO BOTH FACES See Connection Assemblies for more information

## Connection Assemblies



| Connection Detail | 2 Rows of Nails at $122^{\circ}$ oc | 3 Rows of Nails at $12{ }^{\circ}$ oc | $\begin{aligned} & 2 \text { Rows of 1/2" } \\ & \text { Bolts at 24" } 0 \text { " } \end{aligned}$ | $\begin{aligned} & 2 \text { Rows of } 1 / 2^{\prime \prime} \\ & \text { Bolts at } 12^{\prime \prime} 0 \text {. } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: |
| A | 788 | 1182 | 780 | 1560 |
| B | 591 | 887 | 585 | 1170 |
| C | 591 | 887 | 878 | 1755 |
| D | 525 | 788 | 780 | 1560 |
| E | 525 | 788 | 868 | 1736 |
| F | na | na | 520 | 1040 |
| G | na | na | 1560 | 3120 |
| H | Refer to Simpson Strong-Tie catalog for SDS \& SDW installation requirements \& capacities. |  |  |  |

## Notes:

1. When driving nails from each face, alternate every other nail in each row.
2. Unless specifically designed, use $3-1 / 2$ " nails for $1-3 / 4$ " thick plies. If the nails do not fully penetrate the second ply (main member), then the nails shall be driven from both faces.
3. Use 2 rows of nails for depths to $12^{\prime \prime}$. Use 3 rows of nails for depths greater than $12^{\prime \prime}$, up to 18". Use 4 rows of nails for depths greater than 18", up to 24 ".
4. Factored resistances are for standard load duration and shall be adjusted according to code. If the dead load exceeds the live load, the appropriate load duration factor (<1) shall be applied.
5. The Factored Uniform Side-Load Resistance values are the maximum factored load that can be applied to either side of the beam, based on the selected connection detail, and represent loads applied uniformly such as joists supported by hangers spaced 24" oc or less. Connections for discrete point loads may be determined with this table by calculating the equivalent fastener schedule within a 2' length centered about the point load. Details $\mathbf{B}$ and $\mathbf{D}$ shall have the back ply connected with a number of nails equal to half that used to connect the front ply - see the Side-Load Connection Example and detail on page 15. All nail and bolt spacing requirements shall be verified. The full length of the beam shall be connected with the standard connection or with the appropriate side-load connection from this table. The beam shall be designed to support all applied loads.
6. The Factored Uniform Side-Load Resistance for nails is based on 3-1/2" spiral nails for 1-3/4" LVL. For other nail sizes, multiply the Factored Uniform Side-Load Resistance by the Nail Size Factor from the Nail Schedule.
7. The Factored Uniform Side-Load Resistance for bolts is based on ASTM grade A-307, 1/2"0 bolts, for loads applied perpendicular-to-grain (see Fastener Design on page 15).

NAIL SCHEDULE

| Nail Length <br> (in) | Nail Diameter <br> (in) | Lateral Load <br> Capacity (lbs) | Nail Size Factor | Nail Type |
| :---: | :---: | :---: | :---: | :---: |
| $3-1 / \mathbf{2 " ~}^{\prime \prime}$ | 0.160 | 216 | 1.10 | common wire |
|  | 0.152 | 197 | 1.00 | spiral |
| 3 3-1/4" | 0.144 | 178 | 0.90 | common wire |
|  | 0.122 | 131 | 0.66 | spiral |
|  | 0.120 | 127 | 0.64 | power-driven ${ }^{13}$ |
|  | 0.144 | 178 | 0.90 | common wire |
| 3" | 0.122 | 131 | 0.66 | spiral |
|  | 0.120 | 127 | 0.64 | power-driven ${ }^{13}$ |

8. For nails at 8 " 0 c, multiply resistance by 1.5 . For nails at 6 " $o c$, multiply resistance by 2 . For four rows of nails, double the two-row resistance.
9. For detail $\mathbf{A}$, or when attaching the first two plies for detail $\mathbf{B}$ (and optionally for details $\mathbf{F}$ and $\mathbf{H}$ - see note 11), the nails may be driven all from one face or alternating from both faces. If the nails do not fully penetrate the second ply, then the nails shall be driven from both faces.
10. For details $\mathbf{C}$ and $\mathbf{E}$, when side-loaded, the larger side-load shall be applied to the thicker ply (main member).
11. For details $\mathbf{F}$ and $\mathbf{H}$, it is permissible to nail the plies together before bolting or driving Simpson SDS or SDW (or equal) screws. Nail two plies together (see note 8) then nail one additional ply to each side.
12. Beams wider than $5-1 / 2$ " shall be top-loaded or side-loaded from both sides to prevent rotation. For side loads applied to one side of a beam only, the project designer shall verify torsional capacity or detail the beam to prevent rotation due to any side loads. Consult a design professional for other options.
13. Power-driven nails shall have a yield strength equivalent to common wire nails of the same shank diameter.
14. Other nail, screw or bolt configurations are possible. Refer to the Fastener Design table on page 15 or contact your Pacific Woodtech distributor.

## Fastener Design and Fastener \& Load Orientation

FASTENER DESIGN

| Equivalent Specific Gravity |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Nails Only |  | Nails and Wood <br> Screws |  | Bolts and Lag Screws |  |
| Withdrawal |  | Dowel Bearing |  | Dowel Bearing (into the face only) |  |
| Edge | Face | Edge | Face | Load Applied <br> Parallel to Grain |  |
| Perpendicular to Grain |  |  |  |  |  |

## Notes:

1. The equivalent specific gravity for each connection type listed above is for standard load duration and shall be adjusted according to code.
2. Fastener spacing, end and edge distance shall be as specified by code except for nail spacing as specified below.
3. See details to right for fastener and applied load orientation.

## NAIL SPACING REQUIREMENTS

| PWT LVL Ply <br> Thickness | Fastener <br> Orientation |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Nail Size <br> (common wire <br> or spiral) | Minimum <br> End Distance | Minimum Nail Spacing <br> per Row |  |
|  | Edge | Single Row | Multiple Rows |  |  |

## FASTENER \& LOAD ORIENTATION



## Notes:

1. Nails are common wire or spiral nails in accordance with CSA 086.
2. Edge distance shall be such that does not cause spilting.
3. Multiple rows of nails shall be offset at least $1 / 2^{2}$ and staggered.
4. Edge orientation refers to nails driven into the narrow edge of the LVL, parallel to the face of the veneers. Face orientation refers to nails driven into the wide face of the LVL, perpendicular to the face of the veneers. (See Fastener \& Load Orientation details above.)
5. Minimum End Distance and Minimum Nail Spacing are tabulated based on common wire nails. For nails with smaller diameters, the spacing and end distance of the common wire nail with the next larger diameter may be used.
6. Minimum nail spacing is tabulated for LVL stamped with plant number 1089. The minimum spacing may be reduced 1 " for LVL stamped with plant numbers 1066 and 1071.
7. Minimum nail spacing may be reduced 1 " for LVL stamped with plant number 1089, for thickness of 1-3/4" or greater.

## SIDE-LOAD CONNECTION EXAMPLE



Example: Assuming a properly designed 3-ply 14 " beam, determine the equivalent connection to support a factored point load of 6970 lb applied to the side of the beam.

## Solution:

1. Determine the equivalent PLF load over the $2^{\prime}$ 'length by dividing the applied factored load by $2: 6970 \mathrm{lb} / 2^{\prime}=3485$ plf.
2. Divide the equivalent PLF load by the factored uniform side-load resistance for the appropriate detail. For a 14 " depth, 3 rows of nails are required. For detail $\mathbf{B}$ with 3 rows of $3-1 / 2$ " spiral nails at 12 " oc: 3485 plf / $887=3.9$.
3. The required total number of nails is: 3.9 * 3 rows of nails @ 12 " $0 c=11.7$ nails per foot.
4. Connect the front (loaded) ply with the nailing determined in step 3 : drive $123-1 / 2$ " spiral nails within $12^{\prime \prime}$ to each side of the point load (a total of 24 nails). Verify nail spacing.
5. Connect the back ply with half the number of nails determined in step 4 : drive $6,3-1 / 2$ nails, from the back, within 12 " to each side of the point load (a total of 12 nails). Verify nail spacing.
6. Connect full length of member with the standard nailing or as required for side loads.
7. Project designer shall detail to prevent rotation of the beam due to the applied side load.

## HANDLING AND STORAGE GUIDELINES

- WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe structures and possible collapse.
- Keep PWT ${ }^{\text {TM }}$ LVL dry. These products are intended to resist the effects of moisture on structural performance from normal construction delays but are not intended for permanent exposure to the weather.
- Unload products carefully, by lifting. Support the bundles to reduce excessive bowing. Individual products should be handled in a manner which prevents physical damage during measuring, cutting, erection, etc.
- Keep products stored in wrapped and strapped bundles, stacked no more than 10 high. Support and separate bundles with $2 \times 4$ (or larger) stickers spaced no more than 10' apart. Keep stickers in line vertically.
- Product must not be stored in contact with the ground, or have prolonged exposure to the weather.
- Use forklifts and cranes carefully to avoid damaging product.
- Do not use a visually damaged product. Call your local PWT distributor for assistance when damaged products are encountered.
- For satisfactory performance, PWT LVL must be used under dry, covered and wellventilated interior conditions in which the average equilibrium moisture content (MC) of lumber is $15 \%$ or less over a year and does not exceed $19 \%$ at any time. For built-up members, PWT LVL shall be dry before nailing or bolting to avoid trapping moisture.
- PWT LVL shall not be used for unintended purposes such as ramps and planks.




# For product catalog and complete warranty details or for more information on the full 

 line of PWT products or the nearest distributor, visit putewp.com.PWT products are manufactured at different locations in the United States and Canada.

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